Attachment PTA-XV Geologic Maps, Orthogonal Cross-Sections, and Potentiometric Maps Page 1

ATTACHMENT PTA-XV - GEOLOGIC MAPS, ORTHOGONAL CROSS-SECTIONS, POTENTIOMETRIC SURFACE MAPS, and BEDROCK SURFACE MAP

Geologic Map

A geologic map of the site has been prepared, and is provided as **PTA Attachment XV - Geologic Map,** as required by §9 VAC 20-81-460.E.2.c.(4).

The Green Ridge site is underlain by porphyroblastic biotite gneiss (map symbol bgp), which is described as a light-gray, medium grained segregation-layered gneiss containing prominent potassium feldspar porphyroblasts. The minerology includes quartz + biotite + plagioclase + potassium feldspar + muscovite + hornblende. Accessory minerals include epidote, apatite and opaque minerals. It is of Proterozoic age.

Cross Sections

In accordance with §9 VAC 20-81-460.E.2.c.(5), and based on the data collected from the Part A subsurface investigation, five orthogonal cross sections of the site have been prepared and are presented as Figures **PTA Attachment XV - Cross-1 and PTA Attachment XV - Cross-2.**

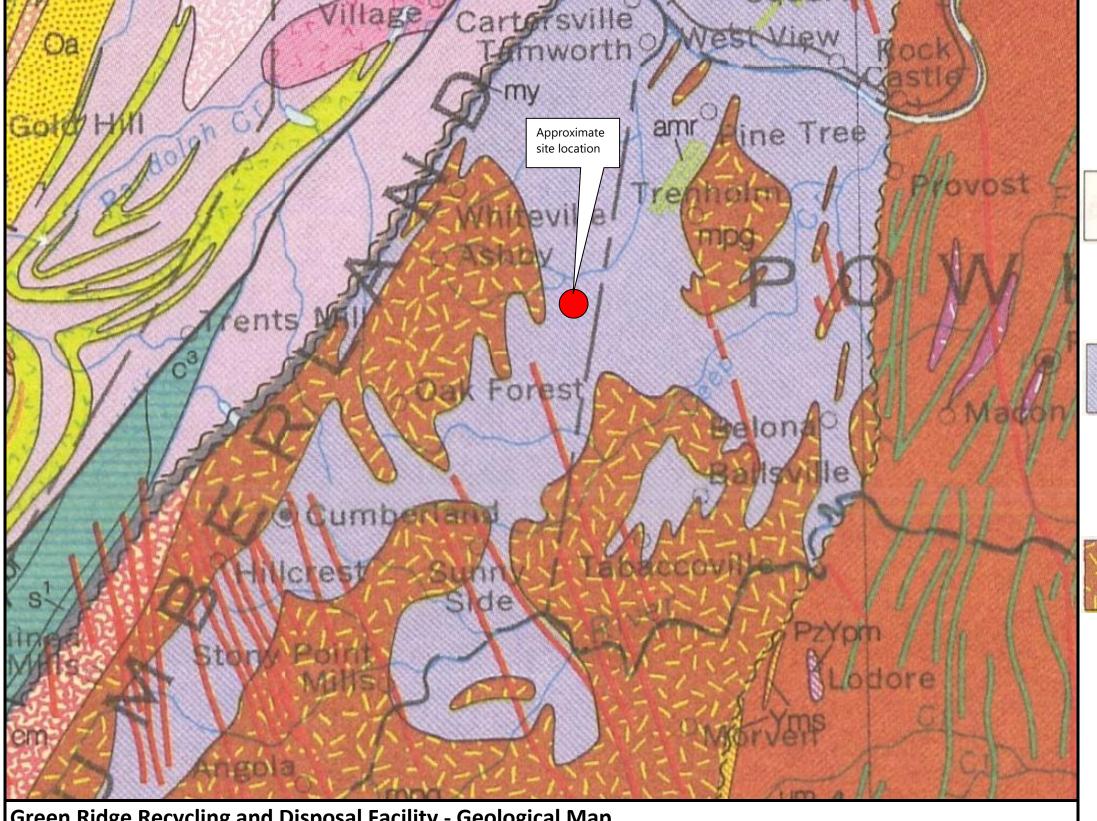
Potentiometric Maps

Potentiometric surface maps of the site have been prepared, based on measurements collected during May and October 2019, and are provided as **PTA Attachment XV - GW-1 and PTA Attachment XV - GW-2** as required by §9 VAC 20-81-460.E.2.c.(6).

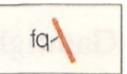
Based on the groundwater elevation data collected during the investigation, potentiometric surface maps were developed that define the groundwater conditions encountered at the proposed solid waste disposal site, based on the stabilized groundwater measurements.

Bedrock Surface Map

A map of the bedrock surface is provided at **PTA Attachment XV-Bed-1**. This shows the configuration of the bedrock surface beneath the site, as it contributes to the overall groundwater flow pattern.



Geologic Legend



Ferruginous Quartzite: Heterogenous layered assemblage correlates with the Chopawamsic Formation and Ta River Metamorphic Suite, on strike to the northeast and traceable into the Milton belt in North Carolina (Geologic Map of North Carolina, 1985). Cambrian Age.



Porphyroblastic Biotite Gneiss: Light-gray, medium grained, segregation-layered gneiss, contains prominent potassium feldspar porphyroblasts. Minerology: quartz + biotite + plagioclase + potassium feldspar + muscovite + hornblende; accessory minerals include epidote, apatite and opaque minerals. Proterozoic age.



Migmatic Paragneiss: Leucocratic to mesocratic, medium- to coarse-grained layered gneiss contains interlayered biotite-rich and quartzofeldspathic zones, locally magmatic; includes lesser amounts of biotite schist, muscovite schist, and thin lenticular amphibolite bodies. Minerology: biotite + muscovite + plagioclase + potassium feldspar + garnet + hornblende. Proterozoic age

Green Ridge Recycling and Disposal Facility - Geological Map

Cumberland County, Virginia Location: Project: Part A Permit Application

Project No. 18020117-030102

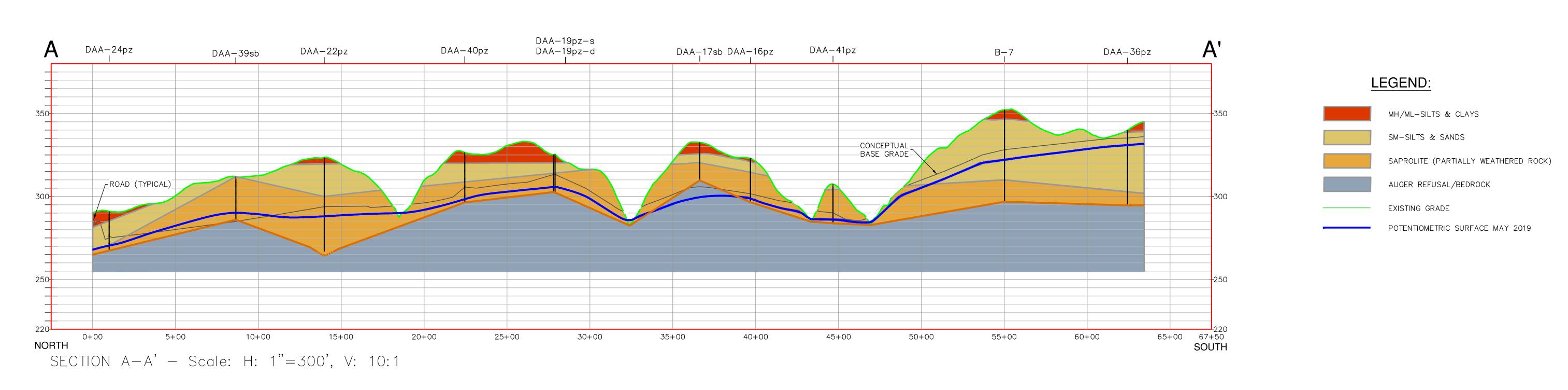


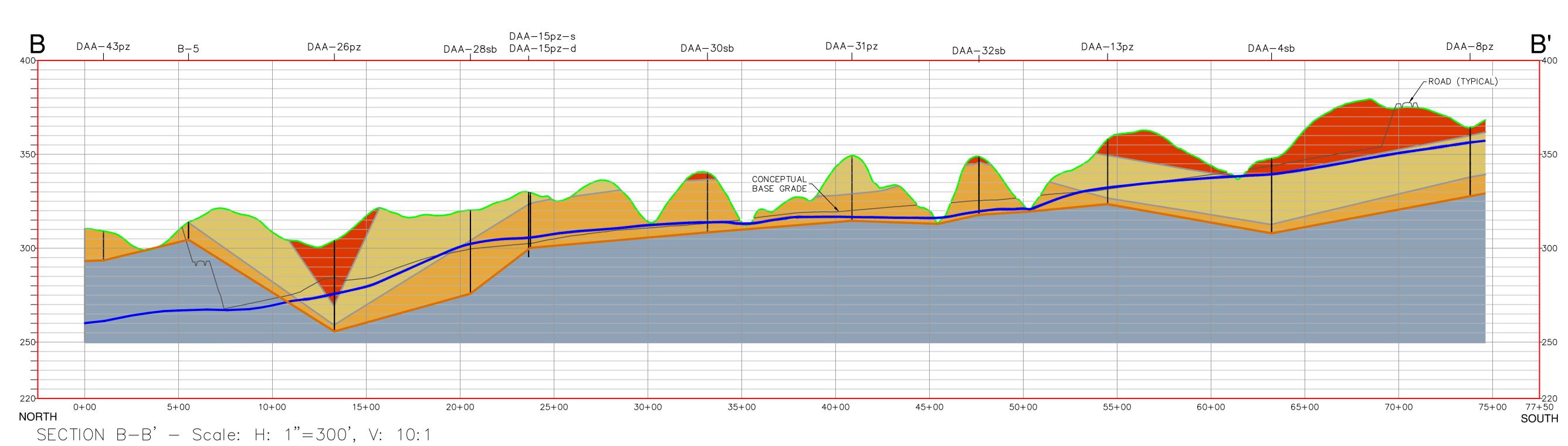
DESIGNED: BHH DRAWN: BHH CHECKED: DC DATE: 12-03-19

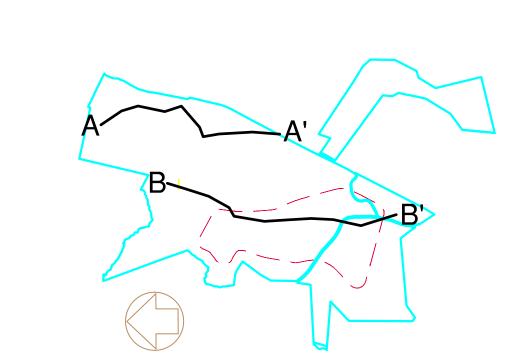
SOURCE: USGS Geologic Map of Virginia, 1993

Scale 1:500,000

FIGURE PTA Attachment XV – Geologic Map







CROSS SECTIONS

SCALE: AS NOTED

KEY MAP SCALE: NOT TO SCALE

No. 2801001751

Associates den

Draper

Engineering



GREEN RIDGE RECYCLING AND DISPOSAL
FACILITY - CROSS SECTION A-A' and B-B'
GREEN RIDGE RECYCLING
AND DISPOSAL FACILITY
CUMBERLAND COUNTY, VIRGINIA

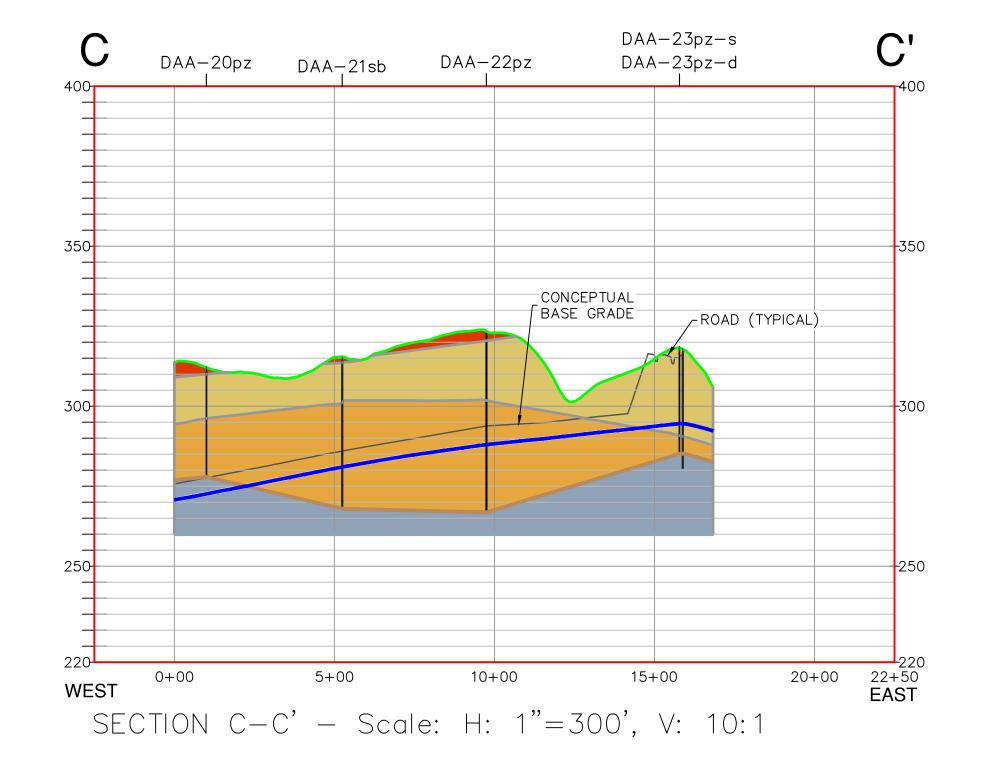
REVISIONS

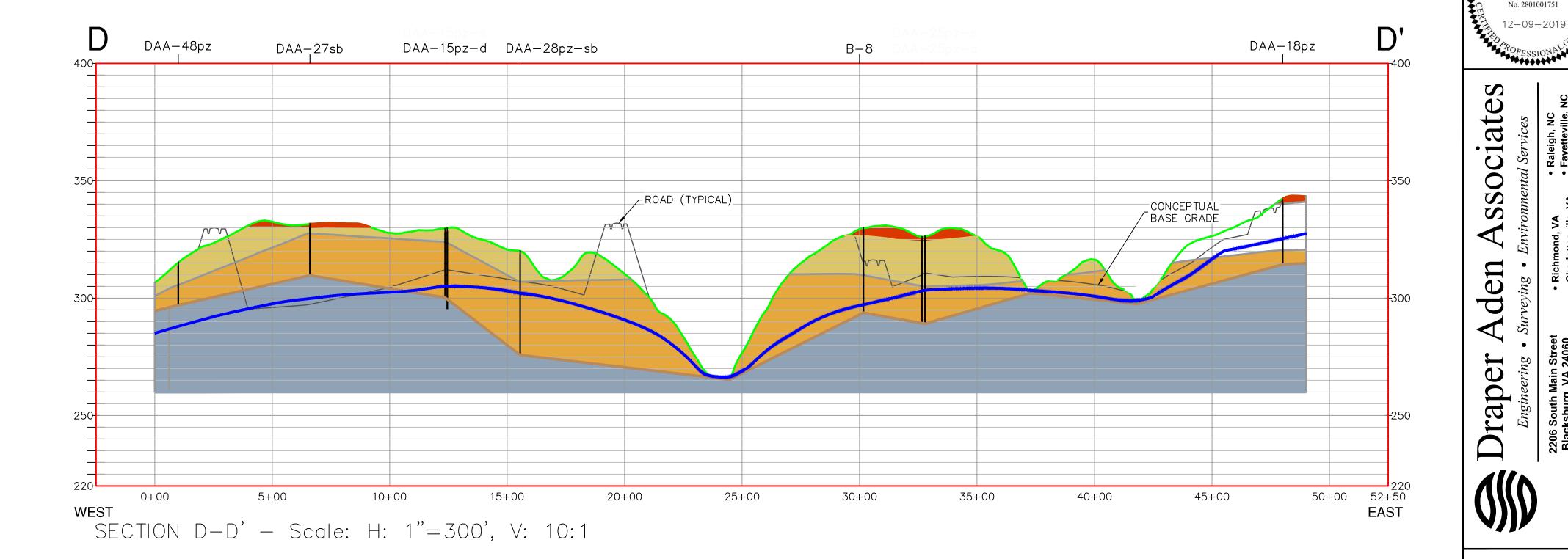
DRAWN BY: DLD/DJF CHECKED BY:

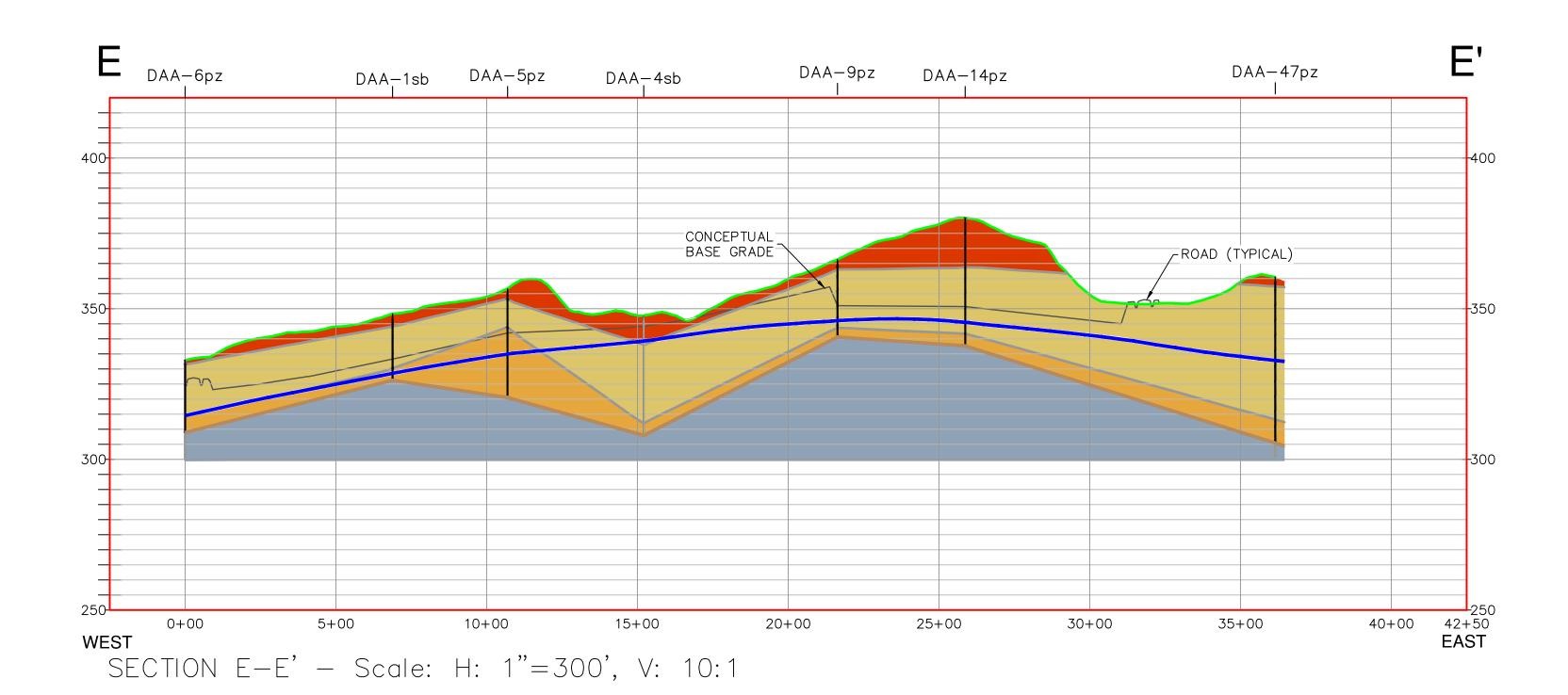
18020117-030102

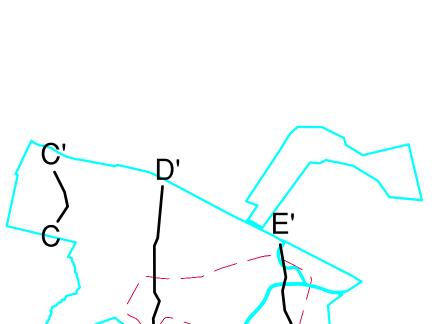
AS NOTED

PTA ATTACHMENT XV FIGURE: CROSS-1













AUGER REFUSAL/BEDROCK

EXISTING GRADE

POTENTIOMETRIC SURFACE MAY 2019

CHECKED BY:

18020117-030102 PTA ATTACHMENT XV FIGURE: CROSS-2

AS NOTED

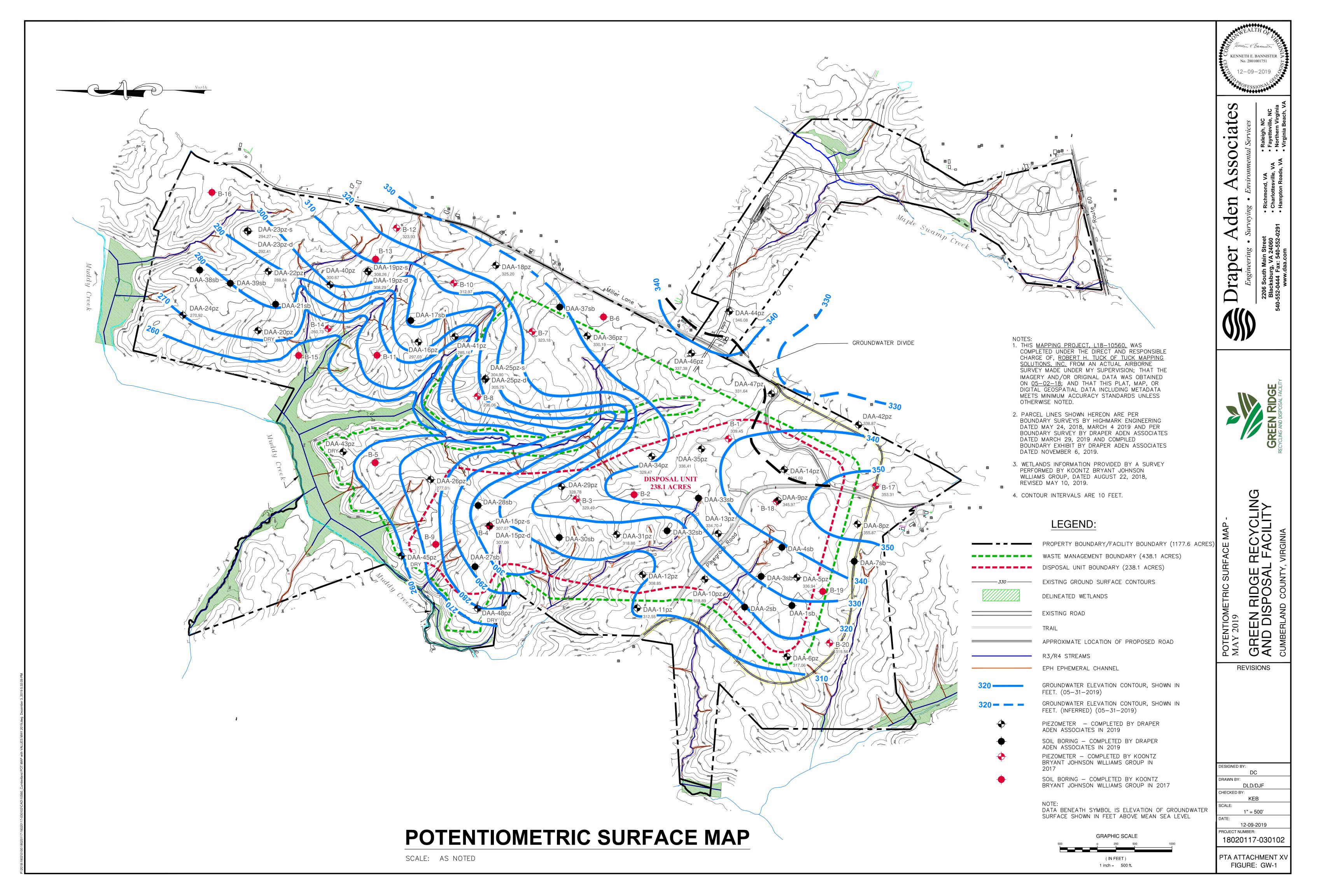
12-09-2019

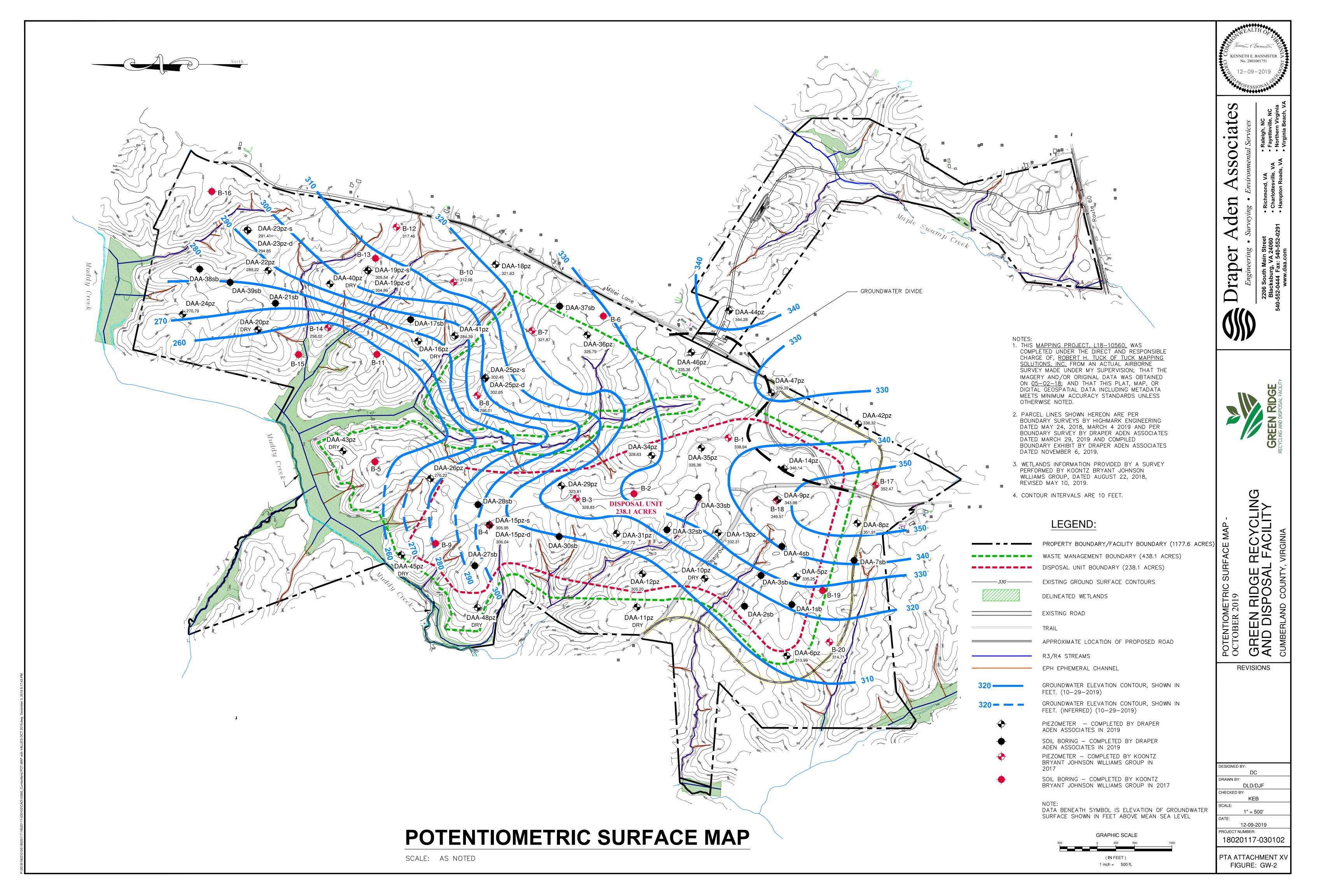
GREEN RIDGE RECYCLING AND DISPOSAL
FACILITY - CROSS SECTIONS C-E
GREEN RIDGE RECYCLING
AND DISPOSAL FACILITY

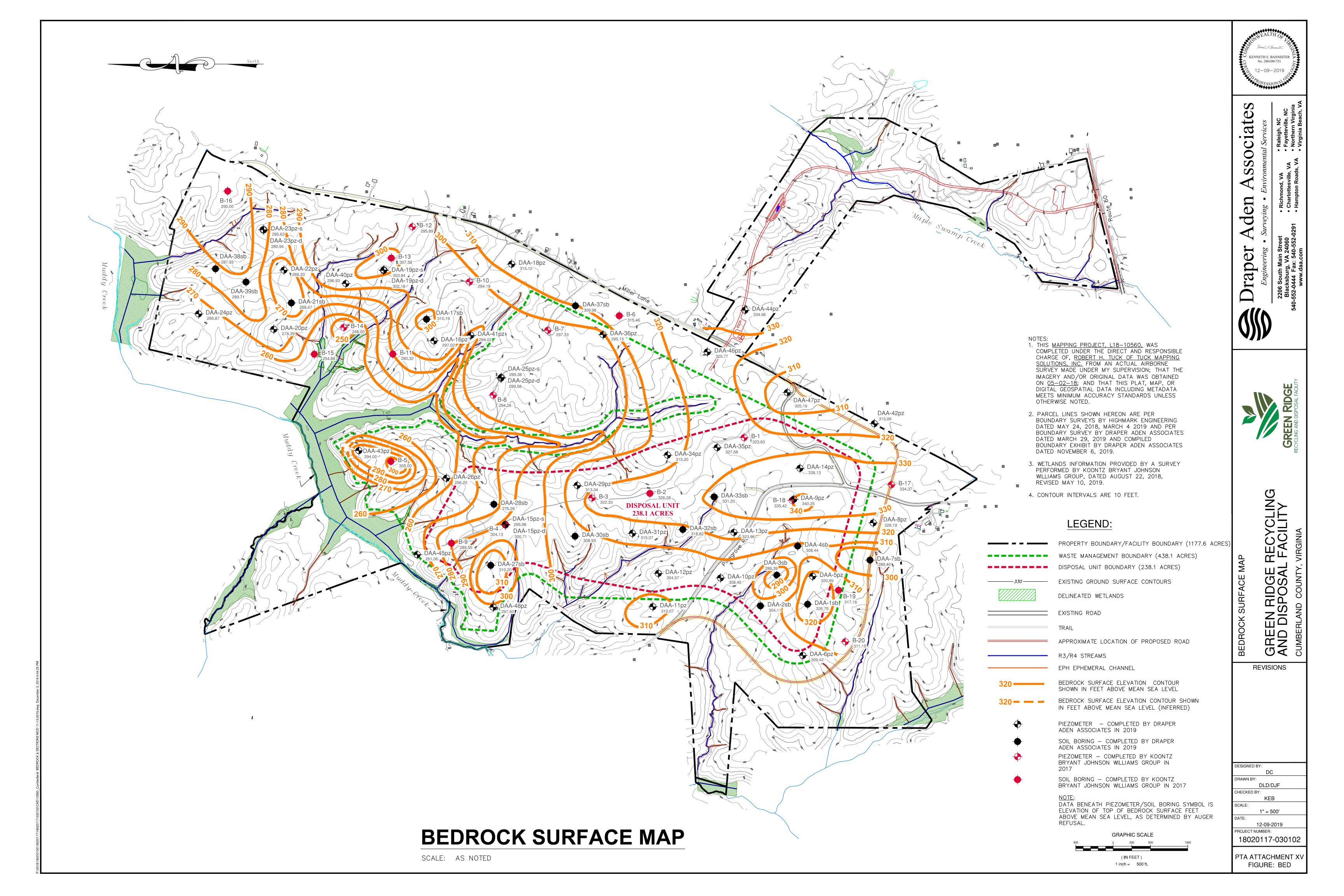
REVISIONS

CROSS SECTIONS

SCALE: AS NOTED







Attachment PTA-XVI VDOT Adequacy Report and Approval Letter Page 1

ATTACHMENT PTA-XVI - VDOT ADEQUACY REPORT AND APPROVAL LETTER

In accordance with §10.1-1408.1.D.1 and 1408.4-.A-1 of the Code of Virginia and §9 VAC 20-81-460 of the *VSWMR*, an application for a new sanitary landfill shall include a written site-specific report approved by the Virginia Department of Transportation (VDOT), evaluating the adequacy of the transportation facilities that will be available to serve the landfill, including the impact of the landfill on local traffic volume, road congestion, and highway safety.



July 3, 2019

Carrie Shepheard, PE
Resident Engineer
VDOT Farmville Residency
637 Commerce Road
Farmville, VA 23901
434-505-3432
Carrie.Shepheard@VDOT.Virginia.gov

RE: Local traffic impact statement for proposed Green Ridge landfill, Cumberland County, VA (DAVENPORT Project Number 183002)

Ms. Shepheard,

We have reviewed the transportation impacts of the proposed Green Ridge Recycling and Disposal Facility to be located on the north side of Route 60 in eastern Cumberland County, Virginia. This memorandum summarizes the proposed site, its transportation-related impacts, and proposed improvements to accommodate future traffic.

Site Description

The proposed landfill site is located on the north side of Route 60 in eastern Cumberland County. Access to the landfill will be served by one (1) proposed access on Route 60 (Anderson Highway) approximately 1,620 feet east of Route 606 (Blenheim Road). This site access is located approximately 1,070 feet west of the Powhatan / Cumberland County border.

Existing Routes 654 and 685 currently run through the site and provide access to several homes located near the site. These routes will not be used by landfill operations. A section of these routes is proposed to be relocated in order to provide area for the southern cell of the landfill. This is conceptually shown in the site exhibit which follows.

Trip Generation

Table 1 on the following page summarizes the trip generation potential for this site. The components of site traffic are discussed below:

Regional Waste Hauling: The hauling of regional waste by large semi-trailer type trucks will be mostly during the nighttime hours: 80% is expected to occur after 6 pm and before 6 am. The remaining 20% will occur during daytime hours (6 am to 6 pm). The projected daily tonnage to be delivered to the site is 3,000 to 5,000 tons. Based on an approximate truck capacity of 20 tons per truck, this represents a maximum of 250 truckloads. Approximately 80% of truckloads will come from the east on Route 60, and 20% from the west on Route 60.

<u>Leachate Hauling:</u> Up to 31 trucks hauling leachate (6,000 to 8,000 gallon tanker trucks) per day may travel to and from the site. Again, 80% are estimated to come from the east on Route 60, and 20% from the west on Route 60. Trucks may arrive during both daytime and nighttime hours.

<u>Local residents:</u> Cumberland County residents will be able to drop off trash at a designated station on the site. Currently, there are three (3) other convenience centers in the County (Madison,



Randolph, and Hamilton Convenience Centers); however, it is understood that the City will be closing at least one of the existing Convenience Centers. The available information indicates approximately four (4) residents per hour may use the facility on weekdays, and a maximum of 30 vehicles per hour on Saturdays. 10 residents per hour on weekdays was assumed, to give more conservative results.

<u>Local garbage trucks:</u> Up to 15 local garbage trucks per day may use the site during daytime hours.

Employees, construction workers, and vendors: Approximately 15 employees may be on site during the daytime hours. Also, construction will occur periodically on site in order to prepare new sub-cells. This will occur approximately every other year, with potentially 15 construction workers on site. During the nighttime hours, approximately five (5) employees may be on site. Finally, up to three (3) vendors per day to the site may be expected.

The resulting trip generation numbers are shown in Table 1 below. <u>These numbers reflect a conservative estimate of peak hour trips in order to give a worst-case scenario.</u>

	Table 1 - Trip Generation for Pro	posed Lan	dfill				
۸۰	verage Weekday Driveway Volumes	24 Hour AM Peak			PM Peak		
Average weekday briveway volunies		Two-Way	Ho	our	Hour		
<u>Land Use</u>	Component	<u>Volume</u>	<u>Enter</u>	<u>Exit</u>	<u>Enter</u>	<u>Exit</u>	
	Regional waste hauling: 80% from 6 pm to 6 am, up to 250 trucks per day	500	4	4	50	50	
	Leachate hauling: Up to 31 trucks per day, daytime and nighttime arrivals	62	1	1	1	1	
Landfill	<u>Local garbage trucks:</u> Up to 15 trucks per day, operating during daytime hours	30	1	1	1	1	
	Local residents (convenience center): Approximately 5 per hour on weekdays	180	0	0	10	10	
	Employees: Approximately 15 daytime, 5 nighttime, 15 during construction	70	30	5	5	30	
	Vendors: Up to 3 per day	6	1	1	1	1	
	Total Trips	848	37	12	68	93	

Main: 336.744.1636; Fax: 336.458.9377

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Traffic Volumes and Turn Lanes

Existing traffic volumes on Route 60 were determined from a 24-hour traffic count collected by VDOT at the Cumberland / Powhatan County line, collected on June 25, 2018. This count indicated an annual average daily traffic (AADT) volume of 5,657 vehicles.

Through volumes were projected at 2% growth per year from the current year 2018, using a tenyear projection for analysis purposes. This growth rate was based on a ten-year review of historical volumes on Route 60 both west and east of the Cumberland / Powhatan County line. The one with the higher growth rate (east of the County line) was selected, yielding a 2% annual growth rate. Left and right turn volumes were computed based on the trip generation numbers shown in Table 1 above and based on expected trip distribution patterns associated with each component of site traffic. The resulting turning movement volumes during AM and PM peak hours are shown in Table 2 below.

Table 2 - Projected Volumes: Intersection of Route 60 and Proposed Access											
	AM P	eak Hour	PM Peak Hour								
	2018 Existing	2028 Future Build	2018 Existing	2028 Future Build							
EB Left	0	18	0	25							
EB Through	367	447	146	178							
WB Through	78	95	334	407							
WB Right	0	20	0	45							
SB Left	0	9	0	57							
SB Right	0	6	0	32							

The need for left and right turn lanes at the proposed access on Route 60 was reviewed based on criteria in Appendix F of the VDOT Road Design Manual. Based on these projected volumes, the proposed access will warrant a right turn taper on Route 60. Volumes are slightly below the threshold for the warranting of a left turn lane; however, a left turn lane is warranted based on high truck volumes and safety considerations. The attached turn lane exhibit proposes a full left turn lane with 200 feet of storage and 200 feet of taper, and a full right turn lane with 200 feet of storage and 200 feet of taper. These turn lanes are expected to enhance traffic flow and safety at this access.

Access Management

According to Appendix F of the VDOT Road Design Manual, for a route such as Route 60, which is a principal arterial with a speed limit of 50 mph or greater, the minimum spacing standard between a full access entrance and other full accesses or unsignalized intersections is 750 feet. The proposed access is located approximately 1,150 feet west from Pine Cove Trail, and approximately 1,620 feet east from Route 606 (Blenheim Road). Therefore, this access satisfies VDOT spacing requirements.

Sight Distance

The adequacy of sight distance at the proposed access was reviewed based on criteria in the VDOT Road Design Manual Appendix F. For Route 60 with a speed limit of 55 mph and a design speed of 60 mph (speed limit plus 5 mph), the required intersection sight distance is 710 feet looking left and 750 feet looking right (from the stop-controlled approach). Based upon preliminary information that will be field-confirmed for the construction plans submittal, the available sight Home Office

Richmond Office:

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John Davenport Engineering, Incorporated 119 Brookstown Ave. Suite PH1 Winston-Salem, NC 27101 Main: 336.744.1636; Fax: 336.458.9377 Richmond Office:
John Davenport Engineering, Incorporated
16003 Continental Boulevard
South Chesterfield, VA 23834
Main: 804.554.0911



distance is approximately 2,000 feet looking left and 2,000 feet looking right. Therefore, sight distance requirements are satisfied.

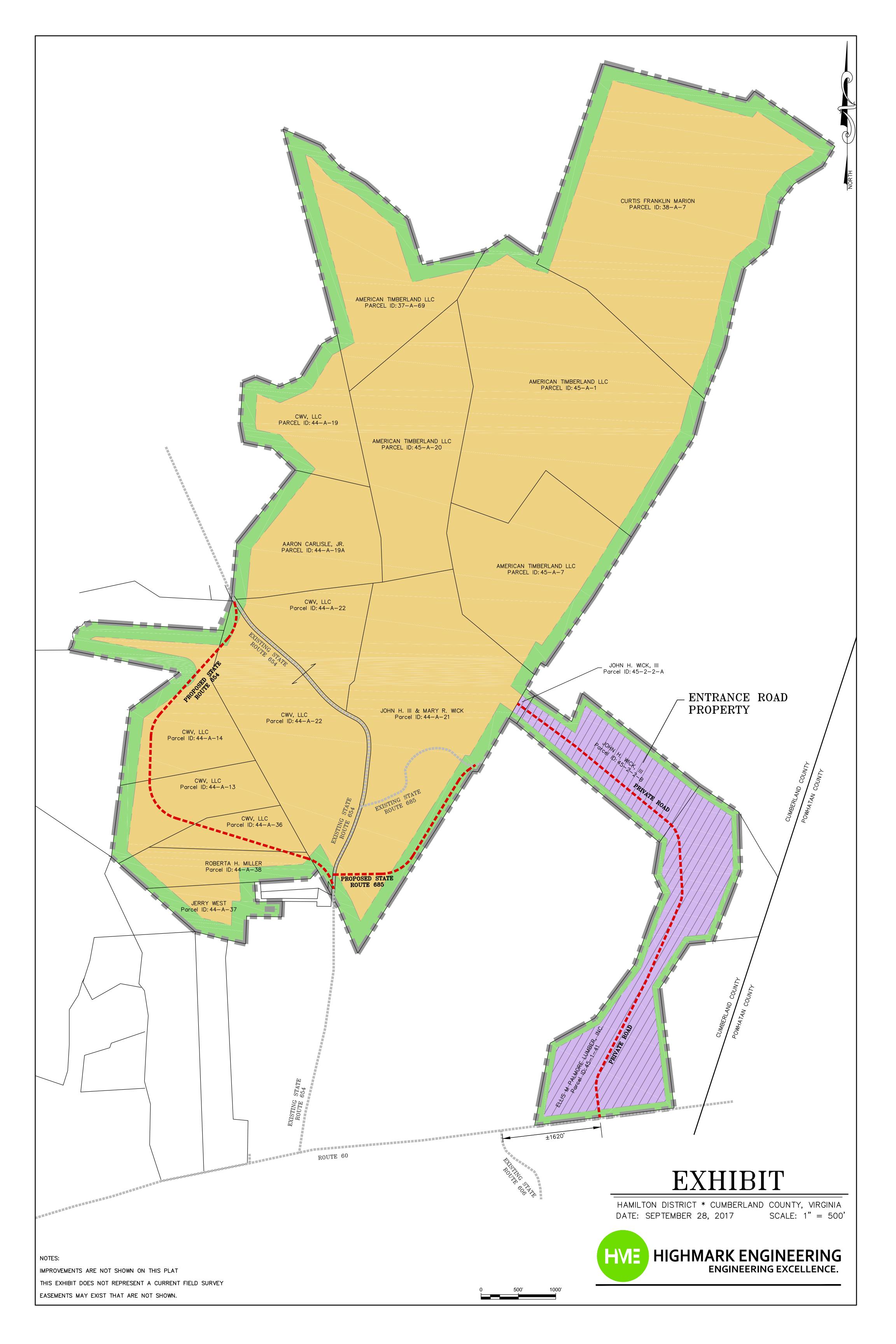
Lighting

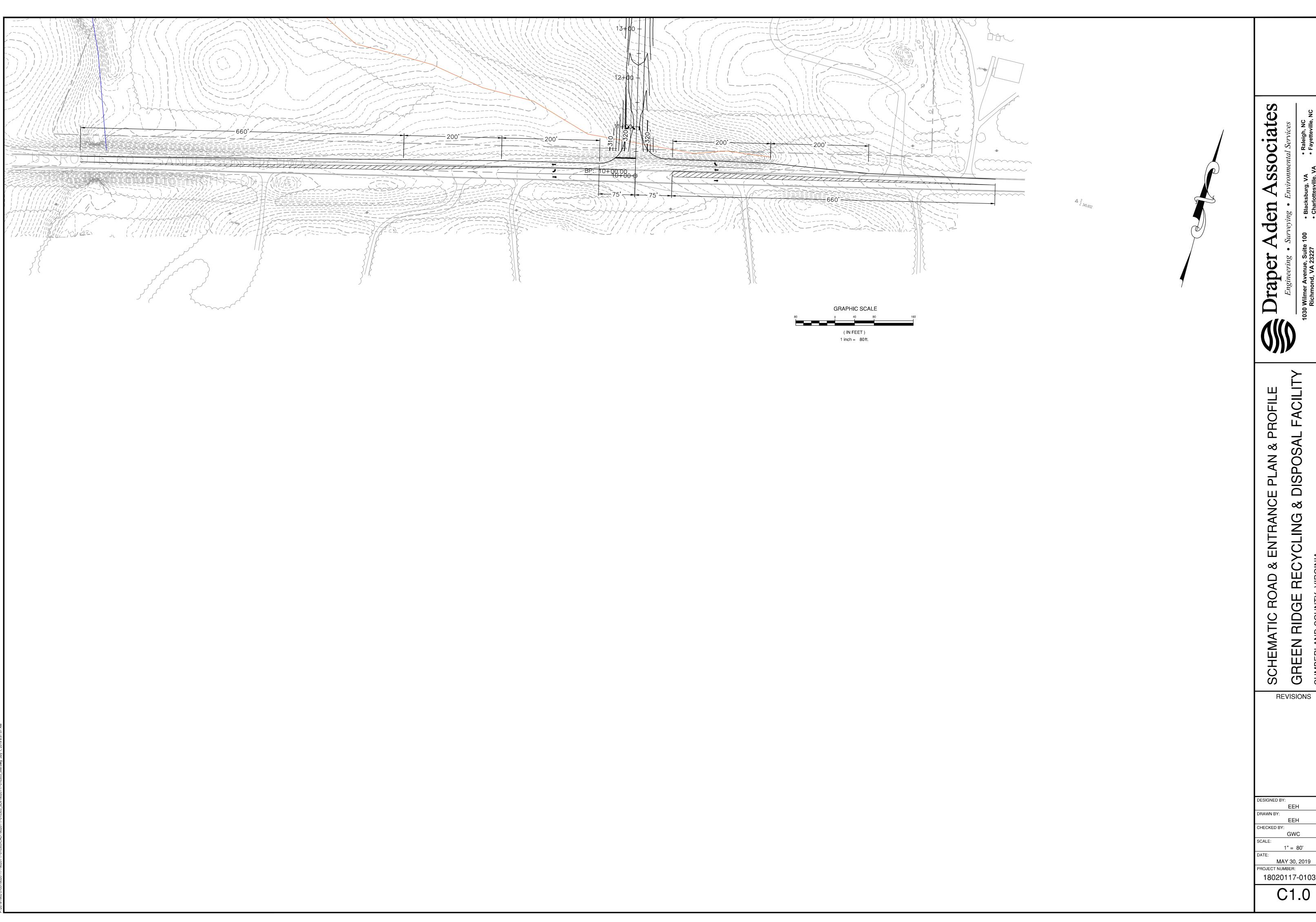
<u>Further review during the development of construction drawings should consider whether streetlights should be installed along Route 60 at this location, given the available sight distances on both the EB and WB approaches (approximately 2,000 feet) and the surrounding roadside environment.</u>

Route Relocations

The site exhibit shows the approximate proposed relocation of a section of Route 654 (Pinegrove Road) and Route 685 (Miller Lane) internal to the site, in order to provide area for the southern cell of the proposed landfill. According to VDOT traffic counts taken in 2014, Route 654 has a volume of approximately 360 vehicles per day south of Route 685 and 270 vehicles per day north of Route 685. Route 685 has a volume of approximately 140 vehicles per day between Pinegrove Road and its dead end at the north. Note that these relocations are shown only conceptually at this stage. The exact design of these roadways and of the relocated intersection of Route 654 and 685 will be determined at the time of construction drawings review.

Additionally, the site access will intersect Route 685 internal to the site. <u>The appropriate configuration for this intersection will be determined at the time of construction drawings review.</u>





FACILIT ISPOSAL

> య GREEN RIDGE RECYCLING VIRGINIA

REVISIONS

EEH

1" = 80'

18020117-010303



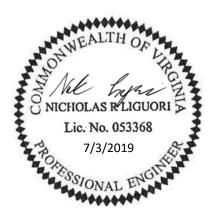
Conclusion

In conclusion, we have reviewed transportation-related impacts of the proposed Green Ridge Recycling and Disposal Facility. Recommendations have been given to provide safe and efficient traffic flow at the site access on Route 60. We recognize that the exact design of the roadway improvements associated with this project will be determined through construction plans review and through further coordination with VDOT and County staff. We look forward to continuing to work with you on this project.

Sincerely,

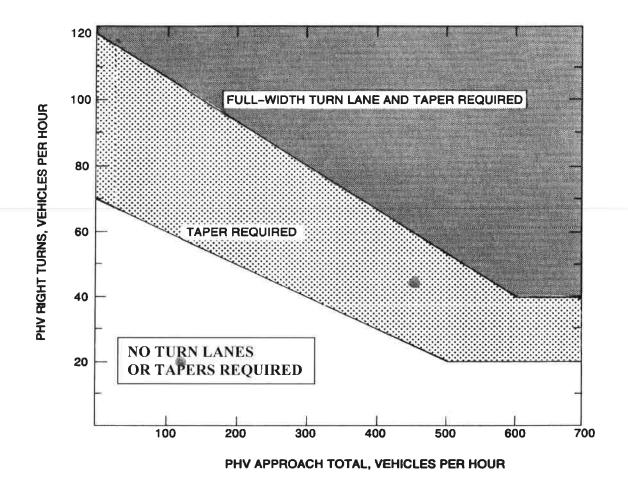
Nick Liguori, PE

Transportation Engineer
John Davenport Engineering, d.b.a. DAVENPORT
(804) 554-0910
nliguori@davenportworld.com





Supporting Documents



Appropriate Radius required at all Intersections and Entrances (Commercial or Private).

LEGEND

PHV - Peak Hour Volume (also Design Hourly Volume equivalent)

Adjustment for Right Turns

For posted speeds at or under 45 mph, PHV right turns > 40, and PHV total < 300.

Adjusted right turns = PHV Right Turns - 20
If PHV is not known use formula: PHV = ADT x K x D

K = the percent of AADT occurring in the peak hour

D = the percent of traffic in the peak direction of flow

Note: An average of 11% for K x D will suffice.

When right turn facilities are warranted, see Figure 3-1 for design criteria. FIGURE 3-26 WARRANTS FOR RIGHT TURN TREATMENT (2-LANE HIGHWAY)

Route 60 at Sik Access VA VR

AM Peak Hour 115 20 => Toper warranted

PM Peak Hour 452 45 (PM peak hour)

WARRANT FOR LEFT-TURN STORAGE LANES ON TWO-LANE HIGHWAY

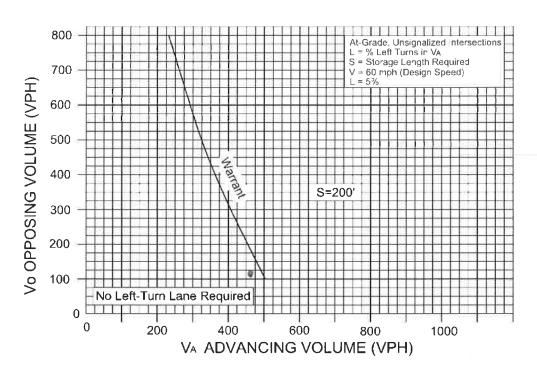
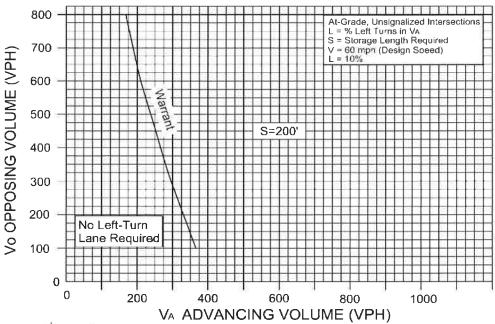


FIGURE 3-17



Route 60 at Sik Hecess VA Vo Y.L. AM Peak Hour 465 115 39

FIGURE 3-18

>> Left turn lane not warranted (but recommended)

WARRANT FOR LEFT-TURN STORAGE LANES ON TWO-LANE HIGHWAY

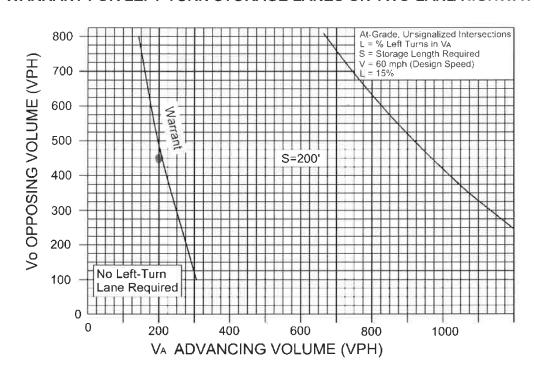
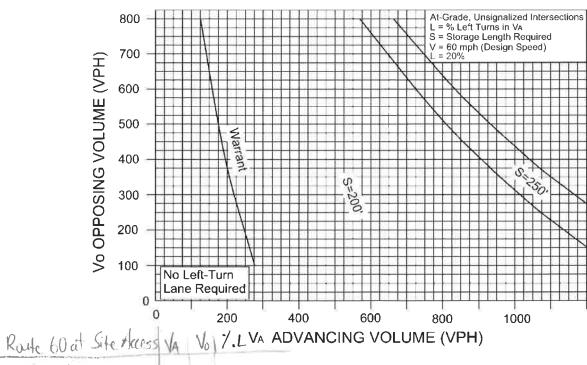


FIGURE 3-19



PM Rak Hour 203 452 12.3 FIGURE 3-20

-> Left turn lane not warrankel (but recommended)

Cumberland County:

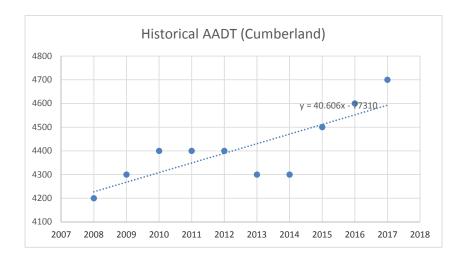
Historical AADT: US 60 between SR 45 and Powhatan County line

2008 2009 2010 2011 2012 2013 2014 2015 2016	4200 4300 4400 4400 4400 4300 4300 4500 4600		
2016 2017	4600 4700		

Fitted trendline:

Slope 40.606 Growth Rate 0.86% Growth Rate

(Rounded) 1.00%



Powhatan County:

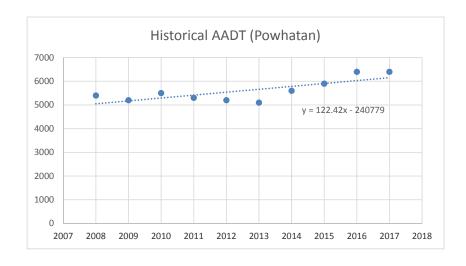
Historical AADT: Cumberland CL to Rte 629 Old Tavern Rd / Trenholm Rd

2008	5400
2009	5200
2010	5500
2011	5300
2012	5200
2013	5100
2014	5600
2015	5900
2016	6400
2017	6400

Fitted trendline:

Slope 122.42 Growth Rate 1.91% Growth Rate

(Rounded) 2.00%



Virginia Department of Transportation Central Region Traffic Engineering

Route: 60 Location: Powhatan/Cumberland Line County: Powhatan Counted By: Ronnie Hall

ADT

ADT 5,657

AADT 5,657

Site Code: Station ID: 60 EBL

WBL

Latitude: 0' 0.0000 Undefined

Start	25-Jur	n-18	Tu	ie	We	ed	Thu		Fr	i	Sa	t	Su	า	Week Ave	erage
Time	EBL	WBL	EBL	WBL	EBL	WBL	EBL	WBL	EBL	WBL	EBL	WBL	EBL	WBL	EBL	WBL
12:00 AM	*	*	11	18	8	17	*	*	*	*	*	*	*	*	10	18
01:00	*	*	7	15	7	12	*	*	*	*	*	*	*	*	7	14
02:00	*	*	11	8	7	6	*	*	*	*	*	*	*	*	9	7
03:00	*	*	25	5	22	6	*	*	*	*	*	*	*	*	24	6
04:00	*	*	36	12	43	12	*	*	*	*	*	*	*	*	40	12
05:00	*	*	162	20	169	23	*	*	*	*	*	*	*	*	166	22
06:00	*	*	366	75	368	80	*	*	*	*	*	*	*	*	367	78
07:00	*	*	277	129	281	148	*	*	*	*	*	*	*	*	279	138
08:00	*	*	213	130	239	150	*	*	*	*	*	*	*	*	226	140
09:00	*	*	164	123	183	116	*	*	*	*	*	*	*	*	174	120
10:00	*	*	160	123	89	58	*	*	*	*	*	*	*	*	124	90
11:00	153	140	143	112	*	*	*	*	*	*	*	*	*	*	148	126
12:00 PM	157	144	130	135	*	*	*	*	*	*	*	*	*	*	144	140
01:00	157	155	160	151	*	*	*	*	*	*	*	*	*	*	158	153
02:00	148	187	170	182	*	*	*	*	*	*	*	*	*	*	159	184
03:00	161	191	167	212	*	*	*	*	*	*	*	*	*	*	164	202
04:00	162	278	159	321	*	*	*	*	*	*	*	*	*	*	160	300
05:00	156	333	137	335	*	*	*	*	*	*	*	*	*	*	146	334
06:00	119	293	102	253	*	*	*	*	*	*	*	*	*	*	110	273
07:00	79	130	78	166	*	*	*	*	*	*	*	*	*	*	78	148
08:00	72	109	48	108	*	*	*	*	*	*	*	*	*	*	60	108
09:00	53	88	36	121	*	*	*	*	*	*	*	*	*	*	44	104
10:00	28	78	19	60	*	*	*	*	*	*	*	*	*	*	24	69
11:00	13	40	11	40	*	*	*	*	*	*	*	*	*	*	12	40
Lane	1458	2166	2792	2854	1416	628	0	0	0	0	0	0	0	0	2833	2826
Day	362		564		204		0		0		0		0		5659	
AM Peak	11:00	11:00	06:00	08:00	06:00	08:00	-	-	-	-	-	-	-	-	06:00	08:00
Vol.	153	140	366	130	368	150	-	-	-	-	-	-	-	-	367	140
PM Peak	16:00	17:00	14:00	17:00	-	-	-	-	-	=	-	-	-	-	15:00	17:00
Vol.	162	333	170	335	-	-	-	-	-	-	-	-	-	-	164	334
Comb. Total	362	24	5	646	2	044	C)		0		0		0	56	59

Virginia Department of Transportation Traffic Engineering Division 2017 Annual Average Daily Traffic Volume Estimates By Section of Route Cumberland Maintenance Area

					Cuili	Denano ivia	intenance P	irea							
Route	Length	AADT	QA	4Tire	Bus		Truck +Axle 1Trai		QC	K Factor	QK	Dir Factor	AAWDT	QW	Year
Cumberland County		From	d			24 (00 %	D- i D d								
644) Forest Hill	1.20	40	R			24-600 Ston	ey Point Rd			NA			NA		03/27/201
044)		To				24-631 Da	venport Rd								
		From				SR 13 Old Bu	ckingham Rd								
(645) Goshen Rd	0.80	300	R							NA			NA		05/25/201
<u> </u>		To From				24-646 Max	eys Mill Rd								
(645) Goshen Rd	1.95	190	R							NA			NA		05/25/201
<u> </u>		To	d			24-654 Frenc									
646) Maxeys Mill Rd	1.69	30	L			24-645 G	oshen Rd			NA			NA		04/02/201
(646) Maxeys Mill Rd	1.03	To	T.			US 60 W, At	nderson Hwy						INA		04/02/201
$\widehat{}$		From				US 60 E, An									
(646) Oak Forest Rd	1.10	30	R							NA			NA		04/02/201
		10	9			SR 45 Cart									
647) Brown Rd	2.50	30	L			24-601 C	linton Rd			NA			NA		04/02/201
(647) Brown Rd	2.30	To	<u> </u>			24-654 Pin	egrove Rd						INA		04/02/201
		From				Dead				\dashv					
648) Parker Rd	0.15	20	R			Beud	Liid			NA			NA		04/02/201
0.0		To	oc			24-616 De	ep Run Rd								
		From	c			SR-45 N, Ca	rtersville Rd								
(649) Tavern Rd	0.10	40	F	96%	1%	1%	1% 1%	0%	С	0.222		0.5	40	F	2017
		To From				24-665 High S	St; Tavern Rd			_					
(649) High St	0.25	160	F	98%	1%	0%	1% 0%	0%	С	0.133		0.5	170	F	2017
<u> </u>		To	d			SR-45 S, Ca	rtersville Rd								
O Dalla Dal	0.10	Prom	<u> </u>	0.40/	10/	Buckingham		00/		0.100		0.004	000	_	0017
650 Belle Rd	2.13	320 Ta	F	94%	1%	2% 24-622 Tre	3% 1%	0%	С	0.103		0.694	320	F	2017
		From				Dead									
(651) Raines Tavern Rd	0.50	60	R			Dead	Liid			NA			NA		03/27/201
(001)		To	d			24-636 C	edar Lane								
		From	ic .			24-632 C	A IRA Rd								
652) CA IRA Rd	0.10	250	R							NA			NA		04/06/201
$\overline{}$		To	c			US 60 And	erson Hwy								
<u> </u>	. =-	From				Dead	End			_					20/01/00/
653 High Hill Rd	1.70	150	R							NA			NA		03/21/201
O 0 1 D1	4.70	From				24-600 F	River Rd			\Rightarrow					00/01/001
653 Cooks Rd	4.70	230 To	R			24 (20 I-1	D 4 - 1 - 1 - D - 4			NA			NA		03/21/201
		From	1			24-638 John 2 24-600 Ston				_					
(654) Sunnyside Rd	0.50	310	L R			24-600 Ston	ey Point Rd			NA			NA		04/11/201
(654) 64,6.66 1.6	0.00	T.0				24 C74 E4	IIII D 4								0 .,, 20 .
(654) Sunnyside Rd	2.60	260 From	R			24-674 Ed	ge Hill Ka			NA			NA		04/11/201
(654) 65111.76166 116		To	1		S	R 13 W, Old I	Buckingham Rd	l		TÎ.					0 .,, 20 .
O		From					uckingham Rd								
654 Frenchs Store Rd	6.00	350	R							NA			NA		05/25/201
$\overline{\bigcirc}$		From				US 60 And	erson Hwy								
654) Pinegrove Rd	0.90	360	R							NA			NA		05/25/201
		From				24-685 M	iller Lane			_					
(654) Pinegrove Rd	1.20	270	R							NA			NA		04/02/201
<u> </u>		To From				24-647 B	rown Rd			\Box					
(654) Pinegrove Rd	0.60	210	R							NA			NA		04/02/201
<u> </u>		To From				24-661 Loc	cust Grove			\Box					
654) Pinegrove Rd	1.50	90	R							NA			NA		04/02/201
<u> </u>		To	9			24-616 De	ep Run Rd								

4/9/2018 13



COMMONWEALTH of VIRGINIA

DEPARTMENT OF TRANSPORTATION

4219 CAMPBELL AVENUE LYNCHBURG, VIRGINIA 24501

STEPHEN C. BRICH, P.E. COMMISSIONER

July 10, 2019

Green Ridge Recycling and Disposal Facility c/o Mr. Jerry Cifor 4 Enterprise Avenue Clifton Park, NY 12065

RE:

Traffic Impact Statement

Green Ridge Landfill, Cumberland County, VA

Dear Mr. Cifor:

In accordance with the Code of Virginia, 10.1-1408.1 D.1. and 10.1-1408.4 A.1., the *Green Ridge Traffic Impact Statement*, by John Davenport Engineering, dated July 3, 2019, and received by the VDOT Farmville Residency Office on July 9, 2019, was reviewed by the Lynchburg District Traffic Engineering Section. The statement accounted for the existing traffic volumes, including the individual/private vehicle drop-offs, as well as the traffic volumes that correlate with the proposed daily disposal limit of 5,000 tons per day delivered primarily for the peak hours of 6pm to midnight.

Based on the review, it is determined that by VDOT minimum standards, only a right turn taper is required on Route 60, but the recommended right and left turn lanes are proposed on Route 60. If DEQ has any questions, they may contact either Matthew L. Conner, P.E., PTOE, Assistant District Traffic Engineer at 434-856-8344 or me at the number below.

Sincerely

Harley E. Joseph, Jr., PE

Transportation & Land Use Director

Lynchburg District

VDOT, 637 Commerce Road, Farmville, Virginia 23901 (434) 505-3447